Analisi dei carichi

$h = L_{max} / 25 = \frac{5.65}{25} = 0.226$ m = 22.6 cm

Campata centrale (h = 24 cm ; s = 4 cm ; h_1 = 20 cm)

- travetti	(0.20 x 0.10 x 1) x 2 = 0.04 x 25 =	1.00 kN/m^2
- soletta	(0.04 x 1 x1) = 0.04 x 25 =	1.00 kN/m^2
- laterizi	(0.20 x 0.40 x 1) x 2 = 0.16 x 8 =	1.28 kN/m^2

peso proprio $\mathbf{g_k = 3.28\ kN/m^2}$

- 3 cm massetto	(0.03 x 1 x 1) = 0.03 x 18 =	0.54 kN/m^2
- 2 cm pavimento	(0.02 x 1 x 1) = 0.02 x 20 =	0.40 kN/m^2
- 1.5 cm intonaco	(0.015 x 1 x 1) = 0.015 x 20 =	0.30 kN/m^2
- incidenza tramezzi		1.00 kN/m^2

sovraccarichi permanenti $\mathbf{g'_k = 2.24\ kN/m^2}$

sovraccarichi accidentali $\mathbf{q_k = 3.00\ kN/m^2}$

Sbalzo (h_{sb} = 20 cm ; s = 4 cm ; h_1 = 16 cm)

- travetti	(0.16 x 0.10 x 1) x 2 = 0.032 x 25 =	0.80 kN/m^2
- soletta	(0.04 x 1 x 1) = 0.04 x 25 =	1.00 kN/m^2
- laterizi	(0.16 x 0.4 x 1) x 2 = 0.128 x 8 =	1.02 kN/m^2

peso proprio $\mathbf{g_{sk} = 2.82\ kN/m^2}$

- 3 cm massetto	(0.03 x 1 x 1) = 0.03 x 18 =	0.54 kN/m^2
- 2 cm pavimento	(0.02 x 1 x 1) = 0.02 x 20 =	0.40 kN/m^2
- 1.5 cm intonaco	(0.015 x 1 x 1) = 0.015 x 20 =	0.30 kN/m^2

sovraccarichi permanenti $\mathbf{g'_{sk} = 1.24\ kN/m^2}$

- ringhiera **F = 0.50 kN/m**

sovraccarichi accidentali $\mathbf{q_{sk} = 4\ kN/m^2}$

H = 1.00 kN/m